

Dans le cadre de la  
Formation « Hydraulique Fondamentale »  
Unité de Formation pour la Performance Industrielle



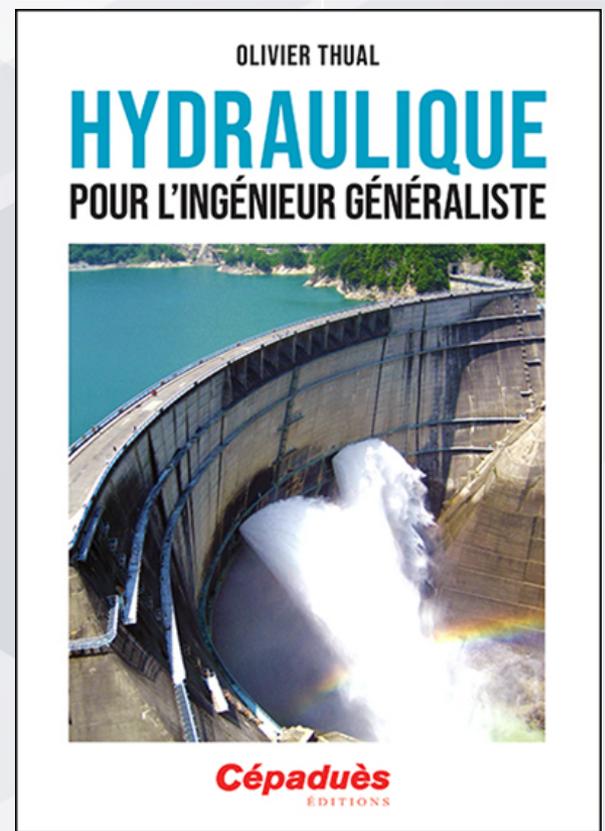
en collaboration avec l'INP-ENSEEIH

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# Chapitre 5

## Hydraulique à surface libre



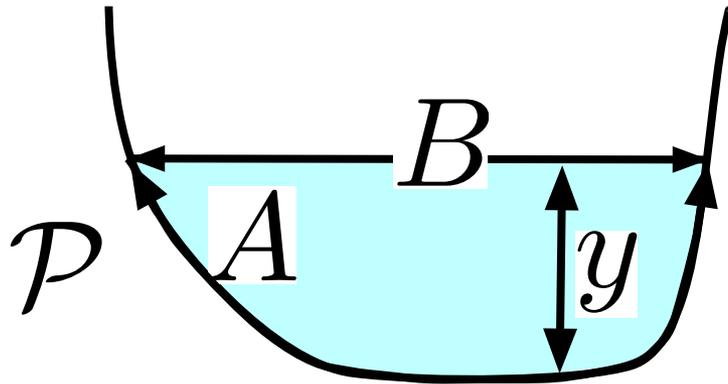
# Canaux et rivières



Charge hydraulique

$$H = \frac{P_a}{\rho g} + Z_f + y + \frac{V^2}{2g}$$

- Aire de la section
- Largeur miroir
- Périmètre mouillé
- Profondeur



Rayon et Diamètre hydrauliques

$$R_H = A / \mathcal{P}$$

$$D_H = 4 R_H$$

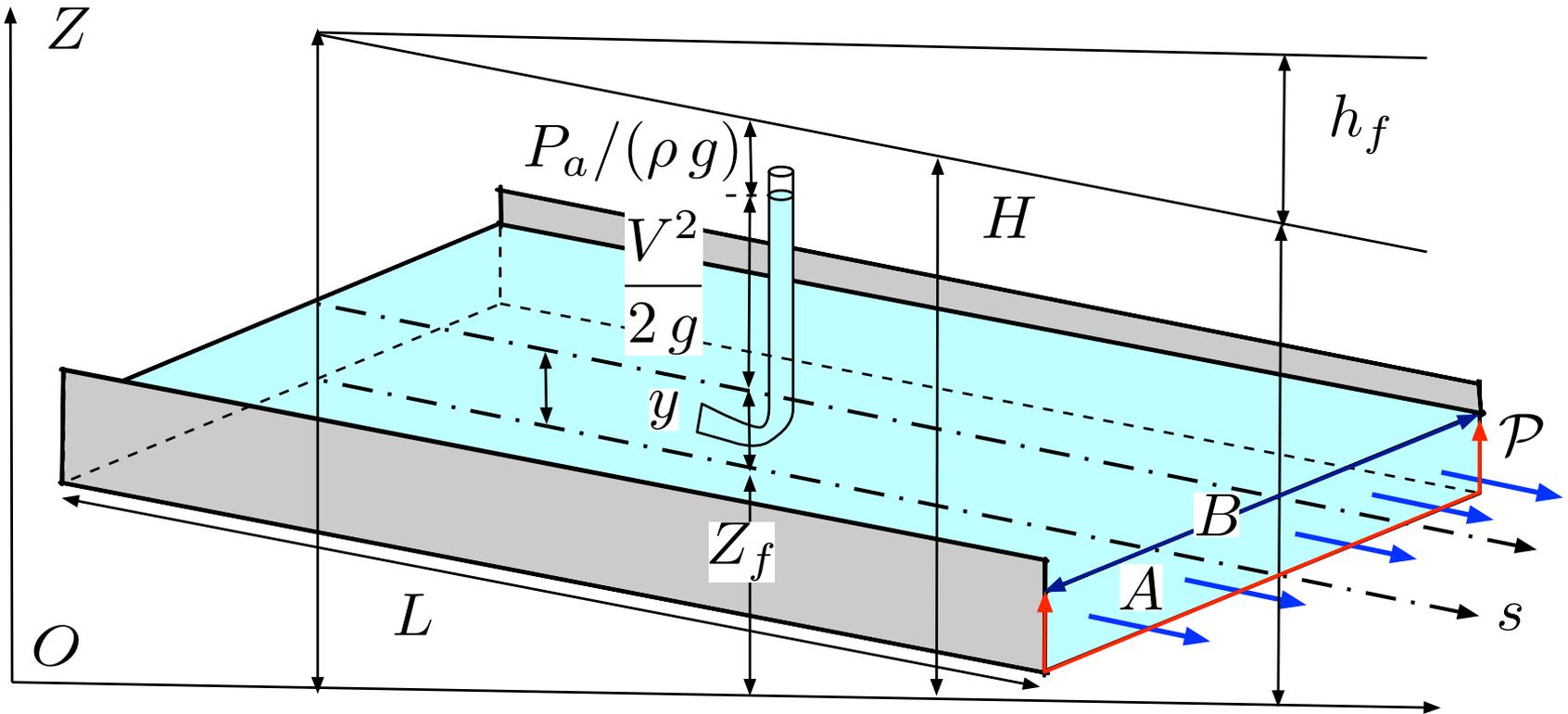
# Pente de frottement

Darcy-Weisbach :

$$S_f = f(r, Re) \frac{V^2}{2g D_H}$$

Pente de frottement

$$S_f = -\frac{dH}{ds} = \frac{h_f}{L}$$

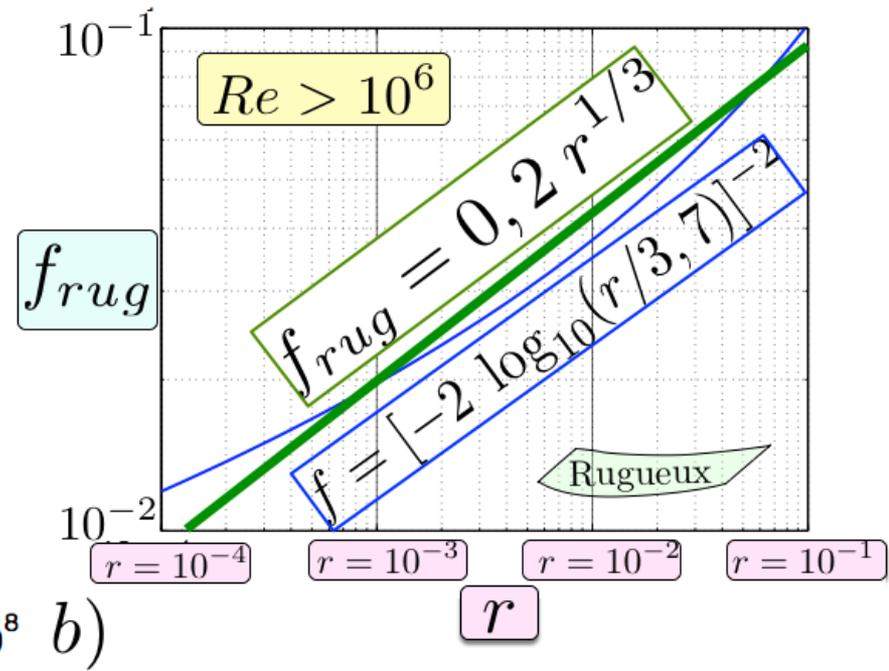
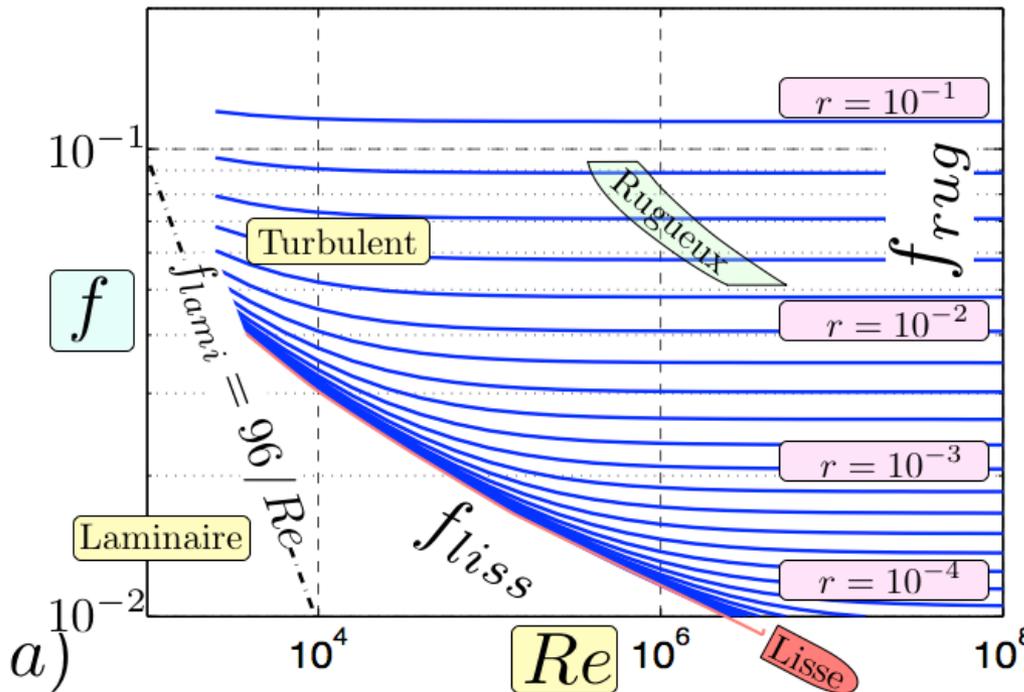


# Régime rugueux

$$\frac{dH}{ds} = S_f = f_{rug}(r) \frac{V^2}{2g D_H}$$

$$r = \frac{\epsilon}{D_H}$$

$$f_{rug}(r) \sim 0,2 r^{1/3}$$



# Formule de Manning-Strickler

$$S_f = f_{rug}(r) \frac{V^2}{2g D_H}$$

$$f_{rug}(r) \sim 0,2 r^{1/3}$$

$$r = \frac{\epsilon}{D_H}$$

$$V = K_s \sqrt{S_f} R_H^{2/3}$$

⇔

$$S_f = \frac{V^2}{K_s^2 R_H^{4/3}}$$

| Fond                     | Strickler $K_s$ | Manning $n$ | Variations |
|--------------------------|-----------------|-------------|------------|
| Ciment lissé             | 90              | 0,011       | 20%        |
| Béton                    | 40              | 0,025       | 30%        |
| Terre                    | 30              | 0,033       | 20%        |
| Herbes ou graviers       | 25              | 0,040       | 25%        |
| Pierres ou broussailles  | 20              | 0,050       | 20%        |
| Mauvaises herbes ou crue | 10              | 0,100       | 40%        |

# Nombre de Froude

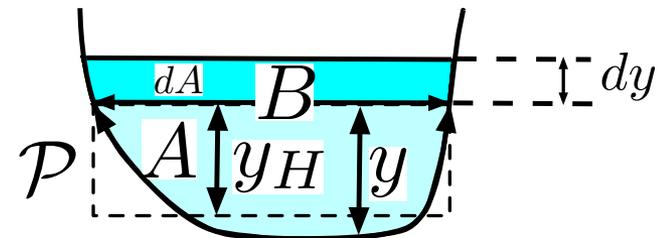
$$dH = -S_f ds \quad \text{avec} \quad H = \frac{P_a}{\rho g} + Z_f + y + \frac{V^2}{2g}$$

$$d(V^2) = d\left(\frac{Q^2}{A^2}\right) = -2 \frac{Q^2}{A^3} dA = -2 V^2 \frac{B}{A} dy$$

$$d\left(\frac{V^2}{2g}\right) = -Fr^2 dy \quad \text{avec} \quad Fr = \frac{V}{\sqrt{g y_H}}$$

$$dA = B dy$$

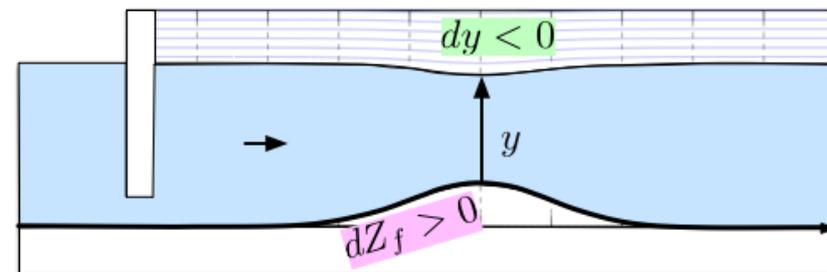
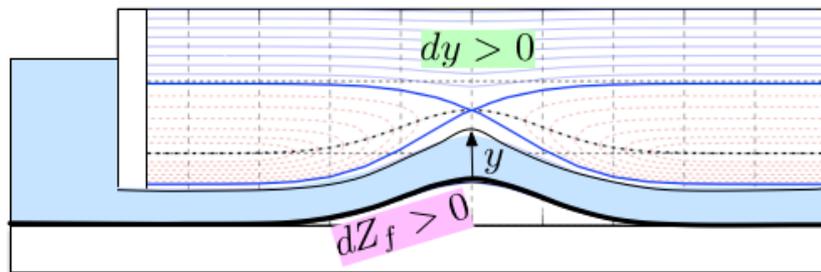
$$y_H = A/B$$



$$(1 - Fr^2) dy = -dZ_f + dH$$

Torrentiel :  $Fr > 1$

Fluvial :  $Fr < 1$



# Courbes de remous

$$\frac{dy}{ds} = \frac{S_0 - S_f}{1 - Fr^2} = S_0 \frac{1 - (y/y_n)^{-10/3}}{1 - (y/y_c)^{-3}}$$

Large canal :  $R_H = y$

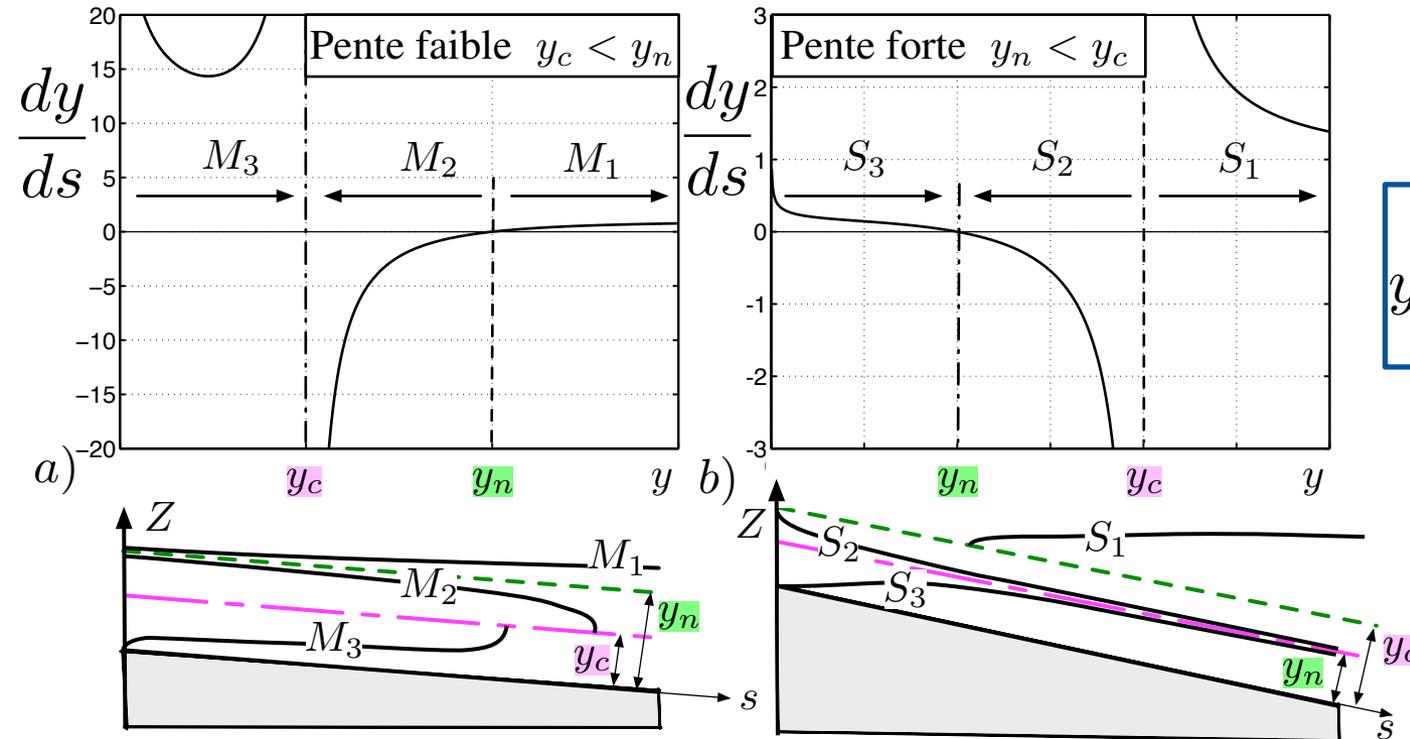
Débit linéique :  $q = \frac{Q}{B}$

Hauteur normale

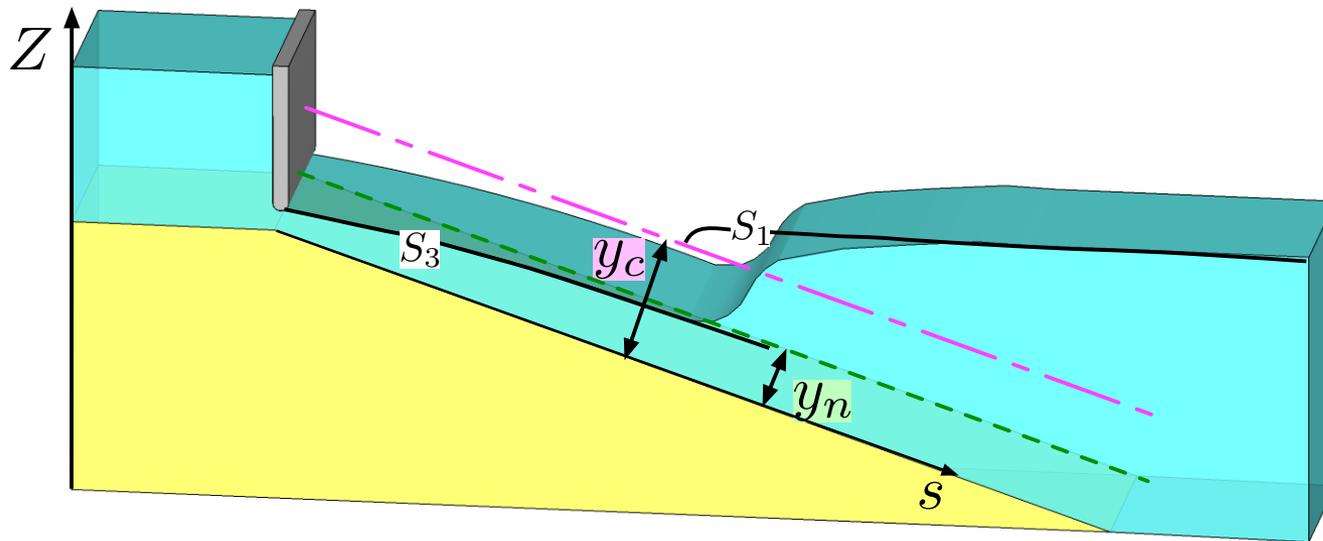
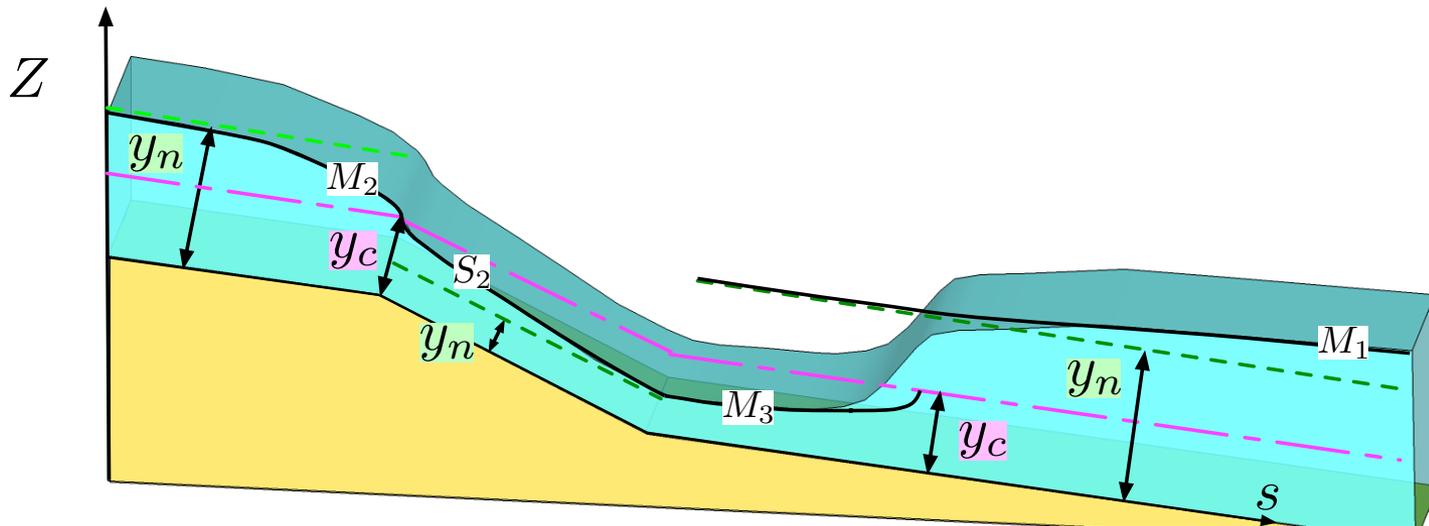
$$y_n = \left( \frac{q^2}{K_s^2 S_0} \right)^{3/10}$$

Hauteur critique

$$y_c = \left( \frac{q^2}{g} \right)^{1/3}$$



# Ressaut hydraulique



# Changement de pente

